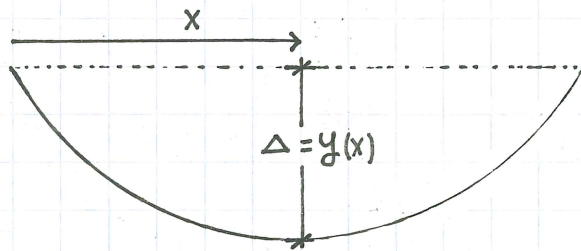


CIVIO2 - STRUCTURES and MATERIALS

Topic: MAT + A + θ

1) $y(x)$	Δ	Displacement	$\int \theta dx$	} Deformations	
$\frac{dy}{dx}$	θ	Slope	$\int \frac{M}{EI} dx$		
$\frac{d^2y}{dx^2}$	ϕ	Curvature	$\frac{M}{EI}$		
$EI \frac{d^2y}{dx^2}$	M	Moment	$\int V dx$	} * EI	
$EI \frac{d^3y}{dx^3}$	V	Shear	$\int W dx$		} Forces
$EI \frac{d^4y}{dx^4}$	W	Distributed Load			

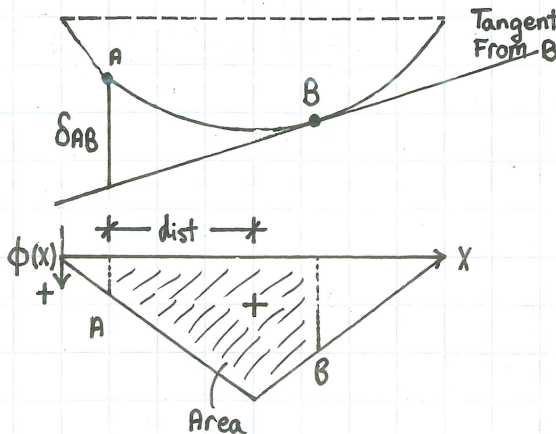


$$\Delta = \iiint \frac{W}{EI} dx$$

2) Moment Area Theorems (MAT)

MAT #1 $\rightarrow \Delta\theta_{AB} = |\theta_B - \theta_A| = \int_A^B \phi(x) dx = \text{Area under } \phi \text{ diagram between A and B}$

MAT #2 $\rightarrow \delta_{DT} = \underbrace{\bar{X}_{DT}}_{\text{Distance}} \underbrace{\int_0^T \phi(x) dx}_{\text{Area}} = \text{Distance between the deflected beam at D and the line that is tangent to T equals the area under the } \phi \text{ diagram times the distance from the centroid of that diagram to D}$



$\delta_{AB} = \text{Area} \cdot \text{dist}$
 A - Where is δ
 B - Where is Tangent
 A - Where is Distance

Note: $\delta_{AB} \neq \delta_{BA}$

Procedure to Find θ and Δ of Beams

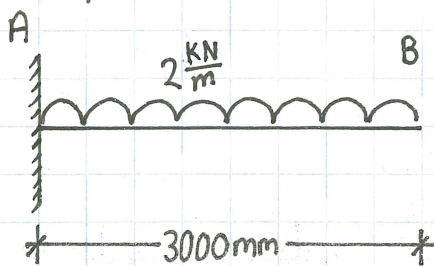
- 1) Calculate the reaction forces, SFD, BMD, ϕ Diagram
- 2) Sketch the displaced shape
 - ↳ The beam can't move up/down at support
 - ↳ The BMD tells you which way it is curving
- 3) Find places where the Δ or θ are known
 - ↳ Supports
 - ↳ Loading Conditions
- 4) Use MAT #1 to find θ
- 5) Use MAT #2 to find Δ

Scenario 1 - Fixed Supports

At fixed support, $\theta = 0$

Example Find θ_B and Δ_B

$$EI \text{ Constant} = 5 \times 10^{13} \text{ Nmm}^2$$



$$\Delta \theta_{AB} = |\theta_B - \theta_A|$$

$$\theta_B = \text{Area under } \phi$$

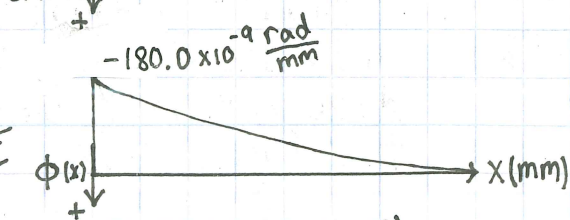
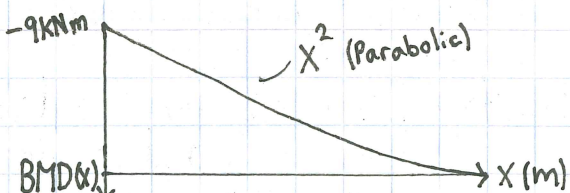
$$= \frac{1}{3}bh$$

$$= \left(\frac{1}{3}\right)(3000 \text{ mm})\left(180.0 \times 10^{-9} \frac{\text{rad}}{\text{mm}}\right)$$

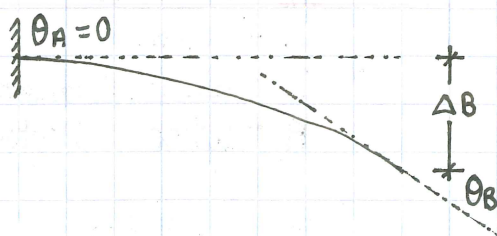
$$\theta_B = 180.0 \times 10^{-6} \text{ rad}$$

$$\delta_{BA} = \Delta_B = (180.0 \times 10^{-6})\left(\frac{3}{4}\right)(3000 \text{ mm})$$

$$\Delta_B = 0.405 \text{ mm}$$



Displaced Shape (NTS)



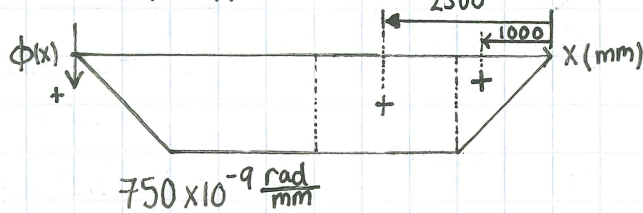
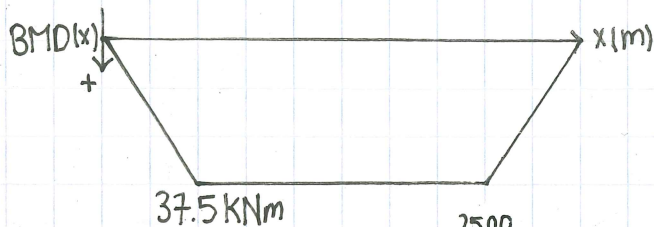
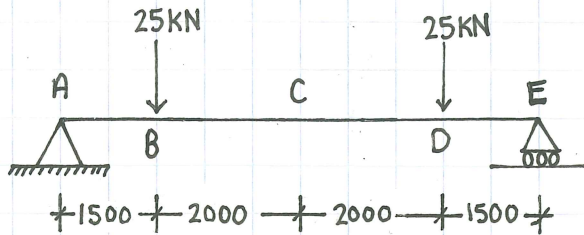
Scenario 2 - Symmetry

If the geometry and loading are symmetrical, then max deflection location is known

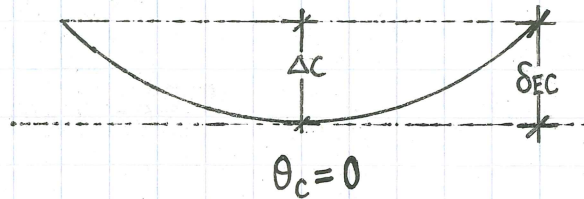
↳ At the location of max Δ , $\theta = 0$

Example Find Δ_C

$$EI = 5 \times 10^{13} \text{ Nmm}^2$$



Displaced Shape



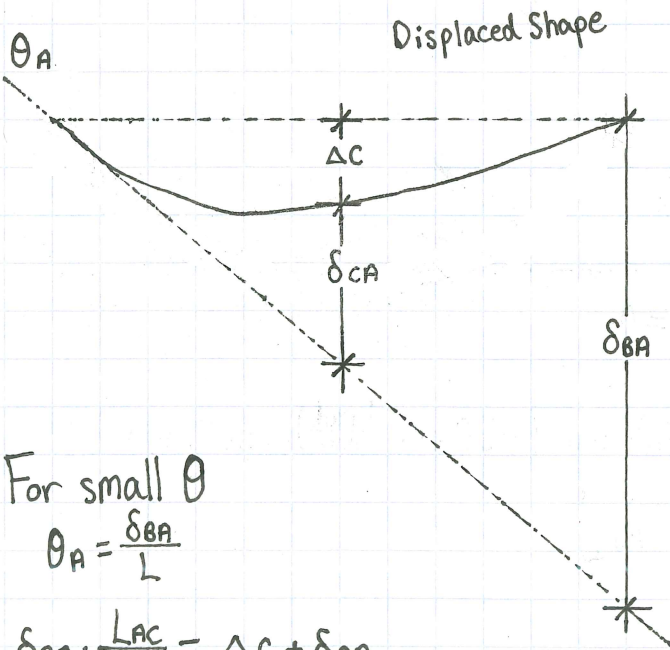
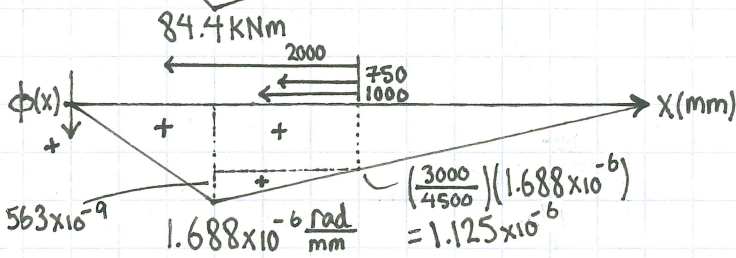
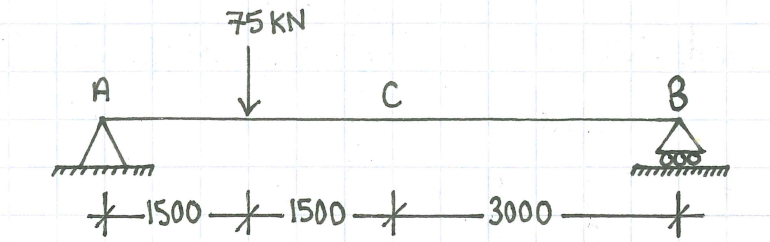
$$\delta_{EC} = \Delta_C = (2000 \text{ mm}) \left(750 \times 10^{-9} \frac{\text{rad}}{\text{mm}} \right) (2500 \text{ mm}) + \left(\frac{1}{2} \right) (1500 \text{ mm}) \left(750 \times 10^{-9} \frac{\text{rad}}{\text{mm}} \right) (1000 \text{ mm})$$

$$\Delta_C = 4.31 \text{ mm}$$

Scenario 3 - Everything Else

No horizontal tangents are known Use similar triangles

Example Find Δ_c $EI = 5 \times 10^{13} \text{ Nmm}^2$



$$\frac{1}{2} \delta_{BA} = \Delta_c + \delta_{CA}$$

$$\delta_{BA} = 17.72 \text{ mm}$$

$$\delta_{CA} = (1500)(1.688 \times 10^{-6})(\frac{1}{2})(2000) + (1500)(1.125 \times 10^{-6})(750) + (1500)(563 \times 10^{-9})(\frac{1}{2})(1000)$$

$$\delta_{CA} = 4.22 \text{ mm}$$

$$\Delta_c = \frac{1}{2} \delta_{BA} - \delta_{CA} = (\frac{1}{2})(17.72) - 4.22$$

$$\Delta_c = 4.64 \text{ mm}$$

For small θ

$$\theta_A = \frac{\delta_{BA}}{L}$$

$$\delta_{BA} \cdot \frac{L_{AC}}{L_{AB}} = \Delta_c + \delta_{CA}$$