

CIVIO2 - STRUCTURES and MATERIALS

Topic: Design of Truss Members

$$1) P_{cr} = \frac{\pi^2 EI}{L^2}$$

- i) Estimate Self Weight
- ii) Calculate the Joint-Loads from Tributary Area
- iii) Calculate Reaction Forces
- iv) Solve for Member Internal Forces $\left\{ \begin{array}{l} \text{Joints} \\ \text{Sections} \end{array} \right.$
- v) Size the Cross Sections of Each Member to Safely Carry the Load

Size?

Tension $F_{fail} = A \cdot f_y$ (force) (Strain)

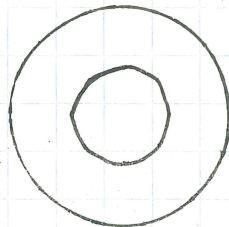
Compression $F_{fail} = A \cdot f_y$

or $F_{fail} = \frac{\pi^2 EI}{L^2}$ (Size term)

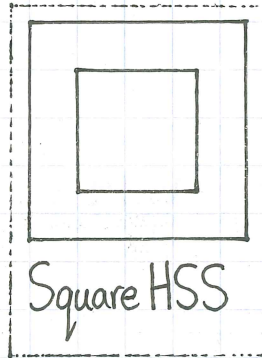
} Smaller will Govern

We Will Use HSS (Hollow Structural Section)

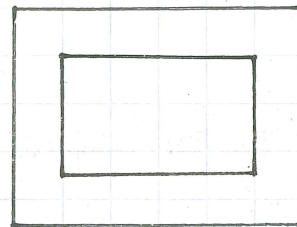
Steel, $E = 200,000 \text{ MPa}$ $f_y = 350 \text{ MPa}$



Circular HSS

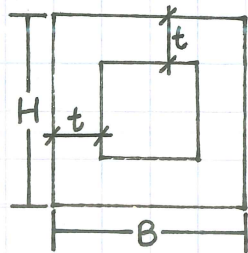


Square HSS



Rectangular HSS

HSS Designation



HSS $H \times B \times t$

Example HSS $305 \times 305 \times 12.7$

Why HSS?

- Light
- Strong (High Second Moment of Area)
- Stiff

I, A Combined

$$r = \sqrt{\frac{I}{A}} \quad \text{radius of gyration}$$

- Geometric Property
- Theoretical Idea

2) Design for Tension

From Method of Joints $\longrightarrow F_{\text{demand}}$

$$F_{\text{capacity}} \geq F_{\text{demand}}$$

$$A \cdot f_y \geq F_{\text{demand}}$$

$$F_{\text{safe}} \geq F_{\text{demand}}$$

$$\frac{A \cdot f_y}{\text{FOS}} \geq F_{\text{demand}}$$

$$\frac{A \cdot f_y}{2} \geq F_{\text{demand}}$$

$$A \geq \frac{2F_{\text{demand}}}{f_y}$$

3) Design for Compression

i) Yield $\longrightarrow A \geq \frac{2F_{\text{demand}}}{f_y}$ Use FOS of 2 against compression yield

ii) Buckling $\longrightarrow P_{\text{cr}} = \frac{\pi^2 E I}{L^2}$ $P_{\text{safe}} = \frac{\pi^2 E I}{\text{FOS} L^2}$

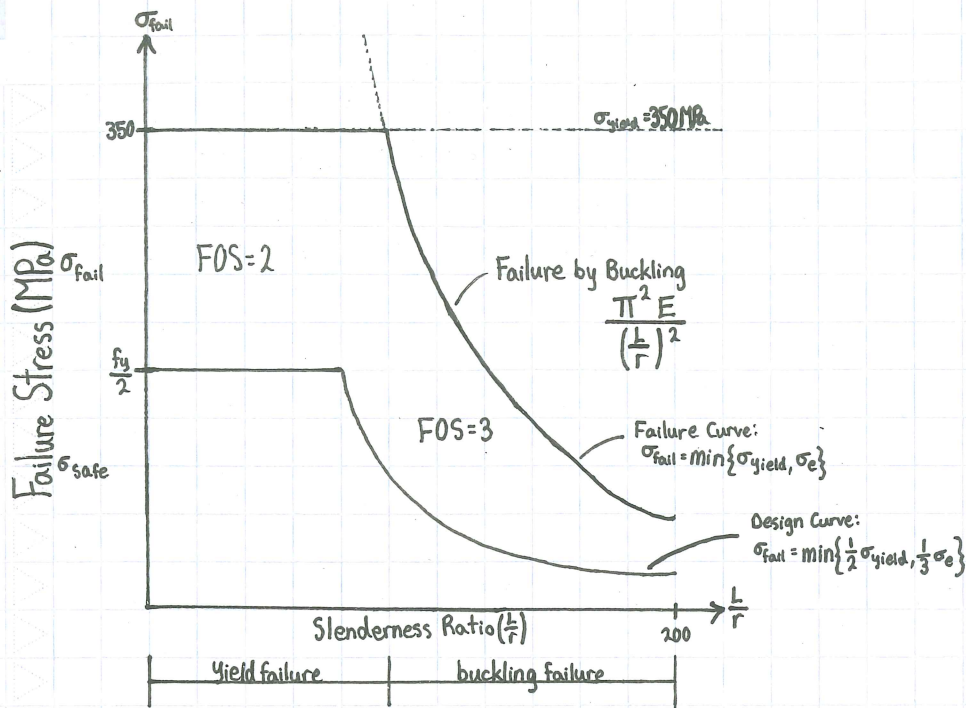
For buckling FOS = 3.0 \longrightarrow $I \geq \frac{\text{FOS} L^2 F_{\text{demand}}}{\pi^2 E}$

Stress at Buckling

$$\frac{P_{\text{cr}}}{A} = \frac{\pi^2 E I}{A \cdot L^2} \quad r = \sqrt{\frac{I}{A}}$$
$$r^2 = \frac{I}{A}$$

$$\sigma_{\text{Euler}} = \frac{\pi^2 E r^2}{L^2}$$

$$= \frac{\pi^2 E}{\left(\frac{L}{r}\right)^2} \quad \frac{L}{r} = \text{Slenderness Ratio}$$
$$\frac{L}{r} \leq 200 \text{ always}$$



Member Type	Cross-Sectional Area, A	Second Moment of Area, I	Radius of Gyration, r
Tension	$\geq \frac{2 \cdot F_{\text{demand}}}{f_y}$	N/A	$\geq \frac{L}{200}$
Compression	$\geq \frac{2 \cdot F_{\text{demand}}}{f_y}$	$\geq \frac{3.0 \cdot F_{\text{demand}} \cdot L^2}{\pi^2 \cdot E}$	$\geq \frac{L}{200}$

Example $F_{\text{demand}} = 250 \text{ kN}$ Compression
 $L = 5000 \text{ mm}$
 $\sigma_y = f_y = 350 \text{ MPa}$

$$A \geq \frac{2 \cdot 250000}{350}$$

$$\geq 1429 \text{ mm}^2$$

$$I \geq \frac{3.0(250000)(5000^2)}{\pi^2(200000)}$$

$$\geq 9.5 \times 10^6 \text{ mm}^4$$

$$r \geq 25 \text{ mm}$$

Select HSS 152 x 152 x 4.8